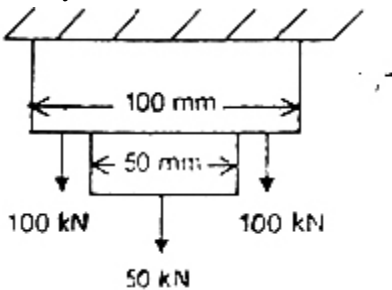


## GATE – 2003

## CE: Civil Engineering

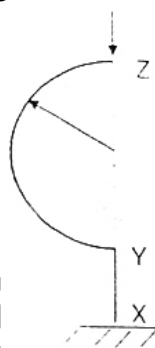
Q.1 – Q.30 Carry one mark each.

1. Given matrix  $[A] = \begin{bmatrix} 4 & 2 & 1 & 3 \\ 6 & 3 & 4 & 7 \\ 2 & 1 & 0 & 1 \end{bmatrix}$ , the rank of the matrix is
- (A) 4 (C) 2  
(B) 3 (D) 1
2. A box contains 10 screws, 3 of which are defective. Two screws are drawn at random with replacement. The probability that none of the two screws is defective will be
- (A) 100% (C) 49%  
(B) 50% (D) None of these
3. If P, Q and R are three points having coordinates (3, -2, -1) (1, 3, 4), (2, 1, -2) in XYZ space, then the distance from point P to plane OQR (O being the origin of the coordinate system) is given by
- (A) 3 (C) 7  
(B) 5 (D) 9
4. A bar of varying square cross-section is loaded symmetrically as shown in the figure. Loads shown are placed on one of the axes of symmetry of cross-section. Ignoring self weight, the maximum tensile stress in  $\text{N/mm}^2$  anywhere is



- (A) 16.0 (C) 25.0  
(B) 20.0 (D) 30.0
5. Muller Breslau principle in structural analysis is used for
- (A) Drawing influence line diagram for any force function

- (B) Writing virtual work equation  
(C) Super-position of load effects  
(D) None of these
6. The effective length of a column in a reinforced concrete building frame, as per IS: 456-2000, is independent of the  
(A) Frame type i.e, braced (no sway) or un-braced (with sway)  
(B) Span of the beam  
(C) Height of the column  
(D) Loads acting on the frame
7. A curved member with a straight vertical leg is carrying a vertical load at Z, as shown in the figure. The stress resultants in the XY segment are



- (A) Bending moment, shear force and axial force  
(B) Bending moment and axial force only  
(C) Bending moment and shear force only  
(D) Axial force only
8. The working stress method of design specifies the value of modular ratio,  $m = 280/(3 \sigma_{abc})$ , where  $\sigma_{abc}$  is the allowable stress in bending compression in concrete. To what extent does the above value of  $m$  make any allowance for the creep of concrete?  
(A) No compensation  
(B) Full compensation  
(C) Partial compensation  
(D) The two are unrelated
9. In the design of lacing system for a built-up steel column, the maximum allowable slenderness ratio of a lacing bar is  
(A) 120  
(B) 145  
(C) 180  
(D) 250

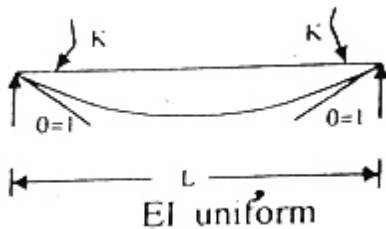
10. Which of the following elements of a pitched roof industrial steel building primarily resists lateral load parallel to the ridge?

- (A) Bracings  
(B) Purlins  
(C) Truss  
(D) Columns

11. Maximum strains in an extreme fibre in concrete and in the tension reinforcement (Fe-415 grade and  $E_s = 200 \text{ kN/mm}^2$ ) in a balanced section at limit state of flexure are respectively

- (A) 0.0035 and 0.0038  
(B) 0.002 and 0.0018  
(C) 0.0035 and 0.0041  
(D) 0.002 and 0.0031

12. The stiffness  $K$  of a beam deflecting in a symmetric mode, as shown in the figure, is



- (A)  $\frac{EI}{L}$   
(B)  $\frac{2EI}{L}$   
(C)  $\frac{4EI}{L}$   
(D)  $\frac{6EI}{L}$

13. A masonry dam is founded on previous sand having porosity equal to 45% and specific gravity of sand particles is 2.65. For a desired factor of safety of 3 against sand boiling, the maximum permissible upward gradient will be

- (A) 0.225  
(B) 0.302  
(C) 1.0  
(D) None of these

14. A double draining clay layer, 6m thick, settles by 30 mm in three years under the influence of a certain loads. Its final consolidation settlement has been estimated to be 120 mm. If a thin layer of sand having negligible thickness is introduced at a depth of 1.5 m below the top surface, the final consolidation settlement of clay layer will be

- (A) 60 mm  
(B) 120 mm  
(C) 240 mm  
(D) None of these

15. A granular soil possesses saturated density of  $20 \text{ kN/m}^3$ . Its effective angle of internal friction is 35 degrees. If the desired factor of safety is 1.5, the safe angle of slope for this soil, when seepage occurs at and parallel to the slope surface, will be

- (A)  $25^0$  (C)  $20^0$   
 (B)  $23^0$  (D)  $13^0$
16. In a plate load test conducted on cohesion less soil, a 600 mm square test plate settles by 15 mm under a load intensity of  $0.2 \text{ N/mm}^2$ . All conditions remaining the same, settlement of a 1 m square footing will be  
 (A) Less than 15 mm (C) 15.60 mm  
 (B) Greater than 25 mm (D) 20.50 mm
17. A 25 kN point load acts on the surface of an infinite elastic medium. The vertical pressure intensity in  $\text{kN/m}^2$  at a point 6.0 m below and 4.0 m away from the load will be  
 (A) 132 (C) 1.32  
 (B) 13.2 (D) 0.132
18. For a two-dimensional irrotational flow, the velocity potential is defined as  $\phi = \ln(x^2 + y^2)$ . Which of the following is a possible stream function,  $\Psi$ , for this flow?  
 (A)  $\frac{1}{2} \tan^{-1}(y/x)$  (C)  $2 \tan^{-1}(y/x)$   
 (B)  $\tan^{-1}(y/x)$  (D)  $2 \tan^{-1}(x/y)$
19. A flat plate is kept in an infinite fluid medium. The fluid has a uniform free-stream velocity parallel to the plate. For the laminar boundary layer formed on the plate, pick the correct option matching Columns I and II.
- | Column I |                                   | Column II                          |
|----------|-----------------------------------|------------------------------------|
| P        | Boundary layer thickness          | 1. Decreases in the flow direction |
| Q        | Shear stress at the plate         | 2. Increases in the flow direction |
| R        | Pressure gradient along the plate | 3. Remains unchanged               |
- Codes:
- | P     | Q | R |
|-------|---|---|
| (A) 1 | 2 | 3 |
| (B) 2 | 2 | 2 |
| (C) 1 | 1 | 1 |
| (D) 2 | 1 | 3 |
20. A laboratory model of a river is built to a geometric scale of 1:100. The fluid used in the model is oil of mass density  $900 \text{ kg/m}^3$ . The highest flood in the river is  $10,000 \text{ m}^3/\text{s}$ . The corresponding discharge in the model shall be  
 (A)  $0.095 \text{ m}^3/\text{s}$  (C)  $0.105 \text{ m}^3/\text{s}$   
 (B)  $0.100 \text{ m}^3/\text{s}$  (D)  $10.5 \text{ m}^3/\text{s}$

21. Water is pumped from a well tapping an unconfined aquifer at a certain discharge rate and the steady state drawdown ( $X$ ) in an observation well is monitored. Subsequently, the pumping discharge is doubled and the steady state drawdown in the same observation well is found to be more than double (i.e, more than  $2X$ ). This disproportionate drawdown is caused by
- (A) Well losses
  - (B) Decrease in the saturated thickness of the aquifer
  - (C) Nonlinear flow
  - (D) Delayed gravity yield
22. The vertical hydraulic conductivity of the top soil at certain is  $0.2$  cm/hr. A storm of intensity  $0.5$  cm/hr occurs over the soil for an indefinite period. Assuming the surface drainage to be adequate, the infiltration rate after the storm has lasted for a very long time, shall be
- (A) Smaller than  $0.2$  cm/hr
  - (B)  $0.2$  cm/hr
  - (C) Between  $0.2$  and  $0.5$  cm/hr
  - (D)  $0.5$  cm/hr
23. The total irrigation depth of water, required by a certain crop in its entire growing period ( $150$  days), is  $25.92$  cm. The culturable command area for a distributary channel is  $100,000$  hectares. The distributary channel shall be designed for a discharge
- (A) Less than  $2$  cumecs
  - (B)  $2$  cumecs
  - (C)  $20$  cumecs
  - (D) More than  $20$  cumecs
24. The moisture content of soil in the root zone of an agricultural crop at certain stage is found to be  $0.05$ . The field capacity of the soil is  $0.15$ . The root zone depth  $1.1$  m. The consumptive use of crop at this stage is  $2.5$  mm /day and there is no precipitation during this period. Irrigation efficiency is  $65\%$ . It is intended to raise the moisture content to the field capacity in  $8$  days through irrigation. The necessary depth of irrigation is
- (A)  $115$  mm
  - (B)  $169$  mm
  - (C)  $200$  mm
  - (D)  $285$  mm
25. The results of analysis of a raw water sample are given below
- |                |   |                   |
|----------------|---|-------------------|
| Turbidity      | : | $5$ mg/l          |
| pH             | : | $7.4$             |
| Fluorides      | : | $2.5$ mg/l        |
| Total Hardness | : | $300$ mg/l        |
| Iron           | : | $3.0$ mg/l        |
| MPN            | : | $50$ per $100$ ml |
- From the data given above, it can be inferred that water needs removal of
- (A) Turbidity followed by disinfection
  - (B) Fluorides and Hardness
  - (C) Iron, followed by disinfection
  - (D) Both (B) and (C)

26. Which of the following sewage treatment methods has inherent problems of odour, ponding, and fly nuisance?  
(A) UASB system (C) Trickling filters  
(B) Activated sludge process (D) Stabilization ponds
27. From amongst the following sewage treatment options, largest land requirements for a given discharge will be needed for  
(A) Trickling filter (C) Oxidation ditch  
(B) Anaerobic pond (D) Oxidation pond
28. Zero hardness of water is achieved by  
(A) Lime soda process (C) Ion exchange treatment  
(B) Excess lime treatment (D) Excess alum and lime treatment
29. Temperature stresses in concrete pavements may cause the slab to crack. If a slab cools uniformly then the crack will develop at the following locations of the slab  
(A) At centre (C) At corners  
(B) Near edges (D) Both (B) and (C)
30. The speed and delay studies on a defined section of highway are conducted by  
(A) Radar gun (C) Moving car method  
(B) Traffic counters (D) Enoscope

**Q.31-90 Carry Two marks each.**

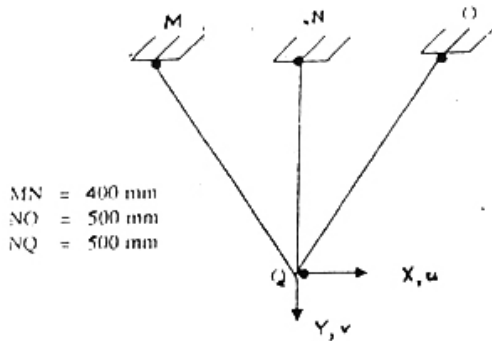
31. If  $L$  defines the Laplace Transform of a function,  $L[\sin(at)]$  will be equal to  
(A)  $a/(s^2 - a^2)$  (C)  $s/(s^2 + a^2)$   
(B)  $a/(s^2 + a^2)$  (D)  $s/(s^2 - a^2)$
32. The Fourier series expansion of a symmetric and even function,  $f(x)$  where  
 $f(x) = 1 + (2x/\pi), -\pi < x < 0$  and  
 $= 1 - (2x/\pi), 0 < x < \pi$  will be  
(A)  $\sum_{n=1}^{\infty} \left(\frac{4}{\pi^2 n^2}\right) (1 + \cos n\pi)$  (C)  $\sum_{n=1}^{\infty} \left(\frac{4}{\pi^2 n^2}\right) (1 - \sin n\pi)$   
(B)  $\sum_{n=1}^{\infty} \left(\frac{4}{\pi^2 n^2}\right) (1 - \cos n\pi)$  (D)  $\sum_{n=1}^{\infty} \left(\frac{4}{\pi^2 n^2}\right) (1 + \sin n\pi)$

33. A long structural column (length =  $L$ ) with both ends hinged is acted upon by an axial compressive load,  $P$ . The differential equation governing the bending of column is given by

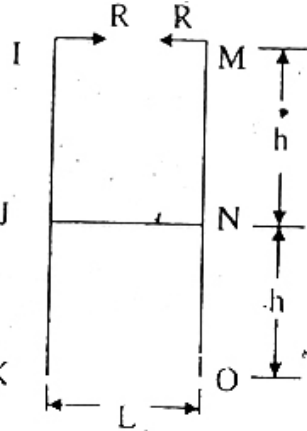
$$EI = \frac{d^2y}{dx^2} = -Py$$

Where  $y$  is the structural lateral deflection and  $EI$  is the flexural rigidity. The first critical load on column responsible for its buckling is given by

- (A)  $\pi^2 EI/L^2$  (C)  $2\pi^2 EI/L^2$   
 (B)  $\sqrt{2}\pi^2 EI/L^2$  (D)  $4\pi^2 EI/L^2$
34. In a redundant joint model, three bar members are pin connected at  $Q$  as shown in the figure. Under some load placed at  $Q$  the elongation of the members  $MQ$  and  $OQ$  are found to be 48 mm and 35 mm respectively. Then the horizontal displacement  $u$  and the vertical displacement  $v$  of the node  $Q$ , in mm, will be respectively.

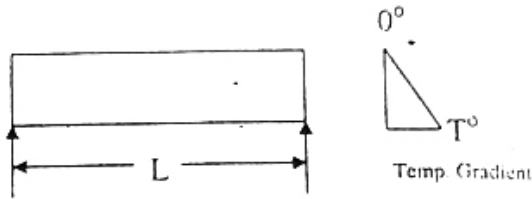


- (A) -6.64 and 56.14 (C) 0.0 and 59.41  
 (B) 6.64 and 56.14 (D) 59.41 and 0.0
35. Top ring beam of an Intze tank carries a hoop tension of 120 kN. The beam cross-section is 250 mm wide and 400 mm deep and it is reinforced with 4 bars of 20 mm diameter of Fe 415 grade. Modular ratio of the concrete is 10. The tensile stress in  $N/m^2$  in the concrete is
- (A) 1.02 (C) 1.20  
 (B) 1.07 (D) 1.32
36. A "H" shaped frame of uniform flexural rigidity  $EI$  is loaded as shown in the figure. The relative outward displacement between points  $K$  and  $O$  is



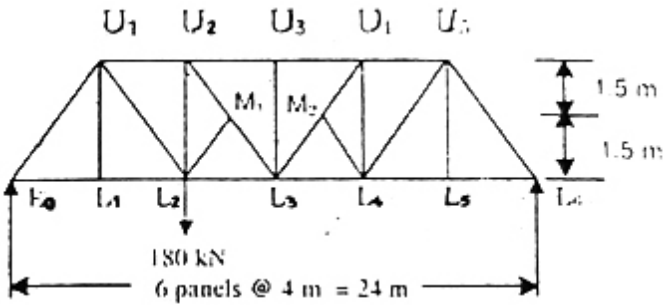
- (A)  $\frac{RLh^2}{EI}$
- (B)  $\frac{RL^2h}{EI}$
- (C)  $\frac{RLh^2}{3EI}$
- (D)  $\frac{RL^2h}{3EI}$

37. A simply supported beam of uniform rectangular cross-section of width  $b$  and depth  $h$  is subjected to linear temperature gradient,  $0^\circ$  at the top and  $T^\circ$  at the bottom, as shown in the figure. The coefficient of linear expansion of the beam material is  $\alpha$ . The resulting vertical deflection at the mid-span of the beam is



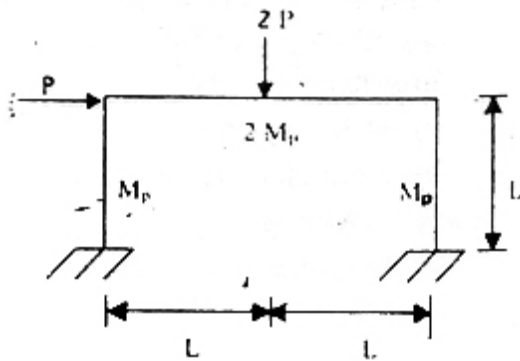
- (A)  $\frac{\alpha Th^2}{8L}$  upward
- (B)  $\frac{\alpha TL^2}{8h}$  upward
- (C)  $\frac{\alpha Th^2}{8L}$  downward
- (D)  $\frac{\alpha TL^2}{8h}$  downward

38. A truss, as shown in the figure, is carrying 180 kN load at node  $L_2$ . The force in the diagonal member  $M_2U_4$  will be



- (A) 100 kN tension
- (B) 100 kN compression
- (C) 80 kN tension
- (D) 80 kN compression

39. A steel portal frame has dimensions, plastic moment capacities and applied loads as shown in the figure. The vertical load is always twice of the horizontal load. The collapse load P required for the development of a beam mechanism is



- (A)  $3M_p/L$
- (B)  $4M_p/L$
- (C)  $6M_p/L$
- (D)  $8M_p/L$

40. The state of two dimensional stresses acting on a concrete lamina consists of a direct tensile stress,  $\sigma_x = 1.5 \text{ N/mm}^2$ , and shear stress  $\tau = 1.20 \text{ N/mm}^2$ , which cause cracking of concrete. Then the tensile strength of the concrete in  $\text{N/mm}^2$  is

- (A) 1.50
- (B) 2.08
- (C) 2.17
- (D) 2.29

41. Group I contains some properties of concrete/cement and Group 2 contains list of some tests on concrete/cement. Match the property with the corresponding test.

Group I

P. Workability of concrete

Group II

1. Cylinder splitting test

- Q. Direct tensile strength of concrete  
 R. Bond between concrete and steel  
 S. Fineness of cement
2. Vee-Bee test  
 3. Surface area test  
 4. Fineness modulus test  
 5. Pull out test

Codes:

	P	Q	R	S
(A)	2	1	5	3
(B)	4	5	1	3
(C)	2	1	5	4
(D)	2	5	1	4

42. Group I contains some elements in design of a simply supported plate girder and Group 2 gives some qualitative locations on the girder. Match the items of two lists as per good design practice and relevant codal provisions.

Group I

- P. flange splice  
 Q. web splice  
 R. bearing stiffeners  
 S. horizontal stiffener

Group II

1. at supports (minimum)  
 2. away from centre of span  
 3. away from support  
 4. in the middle of span  
 5. longitudinally somewhere in the compression

flange

Codes:

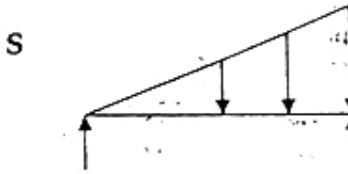
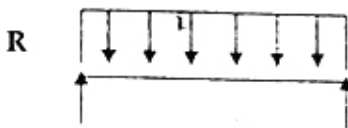
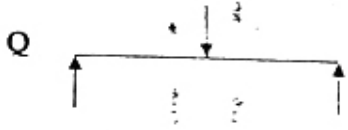
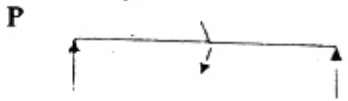
	P	Q	R	S
(A)	2	3	1	5
(B)	4	2	1	3
(C)	3	4	2	1
(D)	1	5	2	3

43. A concrete column carries an axial load of 450 kN and a bending moment of 60 kN m at its base. An isolated footing of size 2m by 3m, with 3m side along the plane of the bending moment, is provided under the column. Centres of gravity of column and footing coincide. The net maximum and the minimum pressures in  $\text{kN/m}^2$  on soil under the footing are respectively.

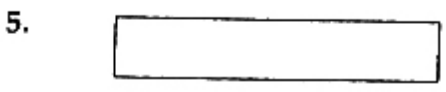
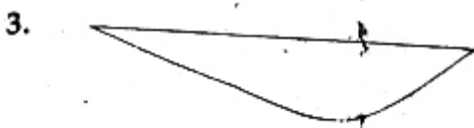
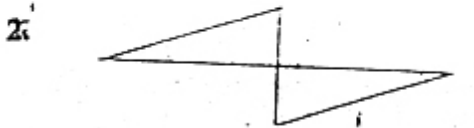
- (A) 95 and 55  
 (B) 95 and 75  
 (C) 75 and 55  
 (D) 75 and 75

44. Group I shows different loads acting on a beam and Group 2 shows different bending moment distributions. Match the load with the corresponding bending moment diagram.

Group 1



Group 2



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Codes:

P	Q	R	S
(A) 4	2	1	3
(B) 5	4	1	3
(C) 2	5	3	1
(D) 2	4	1	3

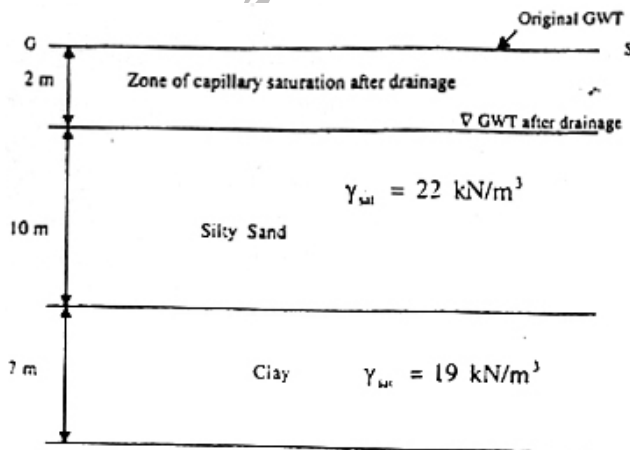
45. Compaction of an embankment is carried out in 500 mm thick layers. The rammer used for compaction has a foot area of 0.05 sq. m and the energy imparted in every drop of rammer is 400 Nm. Assuming 50% more energy in each pass over the compacted area due to overlap, the number of passes required to develop compactive energy equivalent to Indian Standard light compaction for each layer would be

- (A) 10 (C) 20  
(B) 16 (D) 26

46. A braced cut, 5m wide and 7.5 m deep is proposed in a cohesionless soil deposit having effective cohesion  $c' = 0$  and effective friction angle,  $\phi' = 36^\circ$ . The first row of struts is to be installed at a depth of 0.5 m below ground surface and spacing between the struts should be 1.5 m. If the horizontal spacing of struts is 3m and unit weight of the deposit is  $20 \text{ kN/m}^3$ , the maximum strut load will be

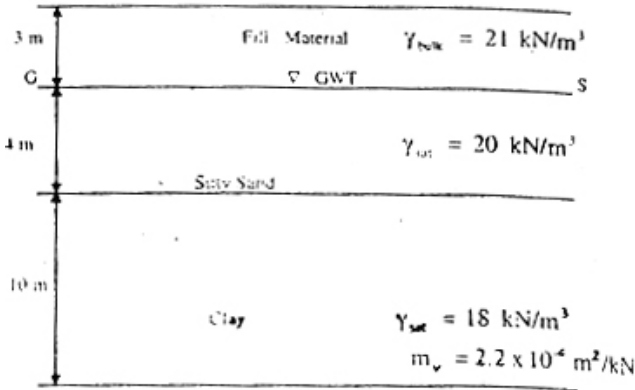
- (A) 70.87 kN (C) 113.90 kN  
(B) 98.72 kN (D) 151.86 kN

47. For the soil strata shown in figure, the water table is lowered by drainage by 2m and if the top 2m thick silty sand stratum remains saturated by capillary action even after lowering of water table, the increase in effective vertical pressure in kPa at mid-height of clay layer will be



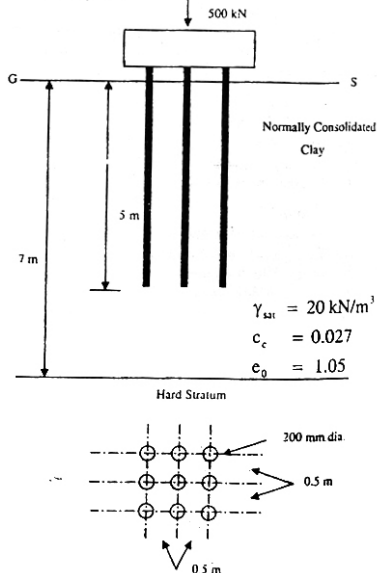
- (A) 0.2 (C) 20  
 (B) 2 (D) 200

48. At a reclamation site for which the soil strata is shown in figure, a 3m thick layer of a fill material is to be laid instantaneously on the top surface. If the coefficient of volume compressibility,  $m_v$ , for clay is  $2.2 \times 10^{-4} \text{ m}^2/\text{kN}$ , the consolidation settlement of the clay layer due to placing of fill material will be



- (A) 69.5 mm (C) 228 mm  
 (B) 139 mm (D) 278 mm

49. For the (3x3) pile group shown in the figure, the settlement of pile group, in a normally consolidated clay stratum having properties as shown in the figure, will be



- (A) 13.2 mm  
(B) 12.775 mm  
(C) 7.345 mm  
(D) None of these

50. Match the items of the two lists and select the correct answer.

List I (Boring Methods)

P. Auger Boring  
rocks

Q. Wash Boring

R. Percussion Drilling

S. Rotary Drilling

Codes:

	P	Q	R	S
(A)	3	1	4	2
(B)	1	2	4	3
(C)	2	3	4	1
(D)	3	1	2	4

List II(Field Conditions)

1. Below water table in all soil types except hard soils and

2. Large diameter boreholes over 150 mm in size

3. Explorations for shallow foundations and highways

4. Bouldery and gravelly strata

51. Match the items of List-I with List-II and select the correct answer.

List I

P. Modulus of subgrade reaction

Q. Relative density and strength

R. Skin friction and point bearing resistance

S. Elastic constants

Codes:

	P	Q	R	S
(A)	1	3	2	5
(B)	1	2	4	3
(C)	2	5	1	3
(D)	3	4	1	2

List II

1. Cyclic pile load test

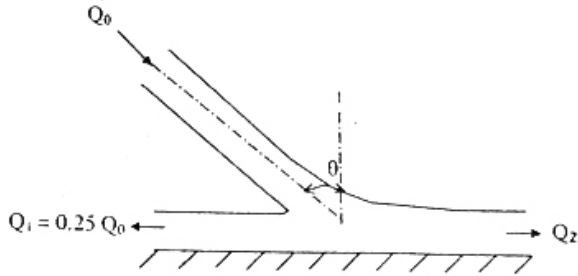
2. Pressure meter test

3. Plate load test

4. Standard penetration test

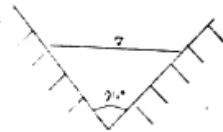
5. Dynamic cone penetration test

52. A horizontal jet strikes a frictionless vertical plate (the plan view is shown in the figure). It is then divided into two parts, as shown in the figure. If the impact loss can be neglected, what is the value of  $\theta$ ?



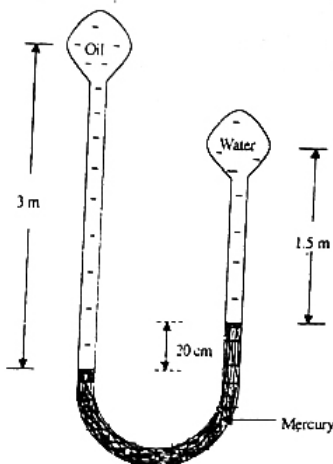
- (A)  $15^\circ$  (C)  $45^\circ$   
 (B)  $30^\circ$  (D)  $60^\circ$

53. A hydraulic jump takes place in a triangular channel of vertex angle  $90^\circ$ , as shown in figure. The discharge is  $1\text{ m}^3/\text{s}$  and the pre-jump depth is  $0.5\text{ m}$ . What will be the post-jump depth? (Take  $g = 9.81\text{ m/s}^2$ )



- (A)  $0.57\text{ m}$  (C)  $1.02\text{ m}$   
 (B)  $0.91\text{ m}$  (D)  $1.57\text{ m}$

54. Two pipelines, one carrying oil (mass density  $900\text{ kg/m}^3$ ) and the other water, are connected to a manometer as shown in the figure. By what amount the pressure in the water pipe should be increased so that the mercury levels in both the limbs of the manometer become equal? (Mass density of mercury =  $13,500\text{ kg/m}^3$  and  $g = 9.81\text{ m/s}^2$ )



- (A) 24.7 kPa (C) 26.7 kPa  
(B) 26.5 kPa (D) 28.9 kPa
55. A solid sphere (diameter 6 mm) is rising through oil (mass density  $900 \text{ kg/m}^3$ , dynamic viscosity  $0.7 \text{ kg/ms}$ ) at a constant velocity of  $1 \text{ cm/s}$ . What is the specific weight of the material from which the sphere is made? (Take  $g = 9.81 \text{ m/s}^2$ )  
(A)  $4.3 \text{ kN/m}^3$  (C)  $8.7 \text{ kN/m}^3$   
(B)  $5.3 \text{ kN/m}^3$  (D)  $12.3 \text{ kN/m}^3$
56. While applying the Rational formula for computing the design-discharge, the rainfall duration is stipulated as the time of concentration because  
(A) This leads to the largest possible rainfall intensity  
(B) This leads to the smallest possible rainfall intensity  
(C) The time of concentration is the smallest rainfall duration for which the Rational formula is applicable  
(D) The time of concentration is the largest rainfall duration for which the Rational formula is applicable
57. While designing a hydraulic structure, the piezometric head at bottom of the floor is computed as 10 m. The datum is 3m below floor bottom. The assured standing water depth above the floor is 2m. The specific gravity of the floor material is 2.5. The floor thickness should be  
(A) 2.00 m (C) 4.40 m  
(B) 3.33 m (D) 6.00 m
58. The plan area of a reservoir is  $1 \text{ km}^2$ . The water level in the reservoir is observed to decline by 20 cm in a certain period. During this period the reservoir receives a surface inflow of 10 hectare meters, and 20 hectare-meters are abstracted from the reservoir for irrigation and power. The pan evaporation and rainfall recorded during the same period at a near by meteorological station are 12 cm and 3 cm respectively. The calibrated pan factor is 0.7. The seepage loss from the reservoir during this period in hectare-meters is  
(A) 0.0 (C) 2.4  
(B) 1.0 (D) 4.6
59. Match the following:
- |                       |                        |
|-----------------------|------------------------|
| Group I               | Group II               |
| P. Rainfall intensity | 1. Isohyets            |
| Q. Rainfall excess    | 2. Cumulative rainfall |
| R. Rainfall averaging | 3. Hyetograph          |

S. Mass curve

4. Direct runoff hydrograph

Codes:

	P	Q	R	S
(A)	1	3	2	4
(B)	3	4	1	2
(C)	1	2	4	3
(D)	3	4	2	1

60. Settling test on a sample drawn from Aeration Tank liquor of ASP (MLSS = 2800 mg/l) was carried out with 1 litre sample. The test yielded a settled volume of 200 ml. The value of Sludge Volume index shall be
- (A) 14.0 (B) 34.2 (C) 71.4 (D) 27.1

61. Results of a water sample analysis are as follows:

Cation	Concentration (mg/l)	Equivalent Weight
Na <sup>+</sup>	40	23
Mg <sup>+2</sup>	10	12.2
Ca <sup>+2</sup>	55	20
+K <sup>+</sup>	2	39

(milliequivalent weight of CaCO<sub>3</sub> = 50 mg/meq).

Hardness of the water sample in mg/l as CaCO<sub>3</sub> is

- (A) 44.8 (B) 89.5 (C) 179 (D) 358
62. An ideal horizontal flow settling basin is 3m deep having surface area 900 m<sup>2</sup>. Water flows at the rate of 8000 m<sup>3</sup>/d, at water temperature 20°C ( $\mu = 10^{-3}$  kg/m.s and  $\rho = 1000$  kg/m<sup>3</sup>). Assuming Stokes law to be valid, the proportion (percentage) of spherical sand particles (0.01 mm in diameter with specific gravity 2.65), that will be removed, is
- (A) 32.5 (B) 67 (C) 87.5 (D) 95.5

63. Match the following:

Group I

(Characteristics of sewage discharged into inland waters)

P. BOD<sub>5</sub>

Group II

(Allowable limit, mg/l)

1. 250

Q. COD	2.30
R. Oil and Grease	3.20
S. Total Suspended Solids	4.10
	5.5
	6.3

Codes:

P	Q	R	S
(A) 2	5	4	2
(B) 4	1	6	4
(C) 3	1	4	2
(D) 2	1	6	3

64. Match the following:

Group I (Type of water impurity)

P. Hardness

Q. Brackish water from sea

R. Residual MPN from filters

S. Turbidity

Group II (Method of treatment)

1. Reverse Osmosis

2. Chlorination

3. Zeolite Treatment

4. Coagulation and Flocculation

5. Coagulation, and Flocculation and

Filtration

Codes:

P	Q	R	S
(A) 1	2	4	5
(B) 3	2	2	4
(C) 2	1	3	5
(D) 3	1	2	5

65. The design speed for a National Highway is 100 kmph. If the maximum permissible super elevation is 0.10 and the coefficient of lateral friction is 0.15, the ruling minimum radius of horizontal curve on the highway should be

(A) 260 m

(B) 315 m

(C) 380 m

(D) 410 m

66. A traffic stream in a particular direction of a two lane road is moving with a constant speed of 50 kmph, with an average headway of 2.52 seconds. The longitudinal distance between two consecutive vehicles is

(A) 30 m

(B) 35 m

(C) 38 m

(D) 42 m

67. In the Marshall method of mix design, the coarse aggregates, fine aggregates, filler and bitumen, having respective specific gravities of 2.62, 2.72, 2.70 and 1.02, are mixed in the ratio of 55, 34.6, 4.8 and 5.6 percent, respectively. The theoretical specific gravity of the mix would be
- (A) 2.36 (C) 2.44  
(B) 2.40 (D) 2.50

68. The plate load test conducted with a 75 cm diameter plate on soil subgrade yielded a deflection of 2.5 mm under a stress of  $800 \text{ N/cm}^2$ . The modulus of elasticity of the subgrade soil, in  $\text{kN/cm}^2$ , is
- (A) 141.6 (C) 160.0  
(B) 154.6 (D) 185.4

69. Column I below gives a list of physical properties of aggregates which should be determined to judge their suitability in road construction. Column II gives a list of laboratory tests which are conducted to determine these properties.

Column I	Column II
P. Hardness	1. Water adsorption
Q. Porosity	2. Impact test
R. Toughness	3. Soundness test
S. Durability	4. Abrasion test

Which of the following matches is correct?

Codes:

P	Q	R	S
(A) 1	2	3	4
(B) 4	1	2	3
(C) 3	4	1	2
(D) 2	3	4	1

**Data for Q.70-72 are given below. Solve the problems and choose correct answers.**

A beam PQRS is 18m long and is simply supported at points Q and R 10m apart. Overhangs PQ and RS are 3m and 5m respectively. A train of two point loads of 150 kN and 100 kN, 5m apart, crosses this beam from left to right with 100 kN load leading.

70. The maximum sagging moment under the 150 kN load anywhere is
- (A) 500 kNm (C) 400 kNm  
(B) 450 kNm (D) 375 kNm

71. During passage of the loads, the maximum and the minimum reactions at support R, in kN, are respectively
- (A) 300 and -30 (C) 225 and -30  
(B) 300 and -25 (D) 225 and -25
72. The maximum hogging moment in the beam anywhere is
- (A) 300 kNm (C) 500 kNm  
(B) 450 kNm (D) 750 kNm

**Date for Q.73-74 are given below. Solve the problems and choose correct answers.**

A reinforced concrete beam, size 200 mm wide and 300 mm deep overall is simply supported over a span of 3m. It is subjected to two point loads P of equal magnitude placed at middle third points. The two loads are gradually increased simultaneously. Beam is reinforced with 2 HYSD bars of 16 mm diameter placed at an effective cover of 40 mm on bottom face and nominal shear reinforcement. The characteristic compressive strength and the bending tensile strength of the concrete are  $20.0 \text{ N/mm}^2$  and  $2.2 \text{ N/mm}^2$  respectively.

73. Ignoring the presence of tension reinforcement, find the value of load P in kN when the first flexure crack will develop in the beam.
- (A) 4.5 (C) 6.6  
(B) 5.0 (D) 7.5
74. The theoretical failure load of the beam for attainment of limit state of collapse in flexure is
- (A) 23.7 kN (C) 28.7 kN  
(B) 25.6 kN (D) 31.6 kN

**Data for Q-75-76 are given below. Solve the problems and choose correct answers.**

A truss tie consisting of 2 ISA  $75 \times 75 \times 8$  mm carries a pull of 150 kN. At ends the two angles are connected, one each on either side of a 10 mm thick gusset plate, by 18 mm diameter rivets arranged in one row. The allowable stresses in rivet are  $f_s = 90.0 \text{ N/mm}^2$  and  $f_{br} = 250 \text{ N/mm}^2$ .

75. Maximum tensile stress in the tie in  $\text{N/mm}^2$  is
- (A) 93.6 (C) 77.2  
(B) 87.5 (D) 66.0
76. Minimum number of rivets required at each end is
- (A) 2 (C) 4  
(B) 3 (D) 5

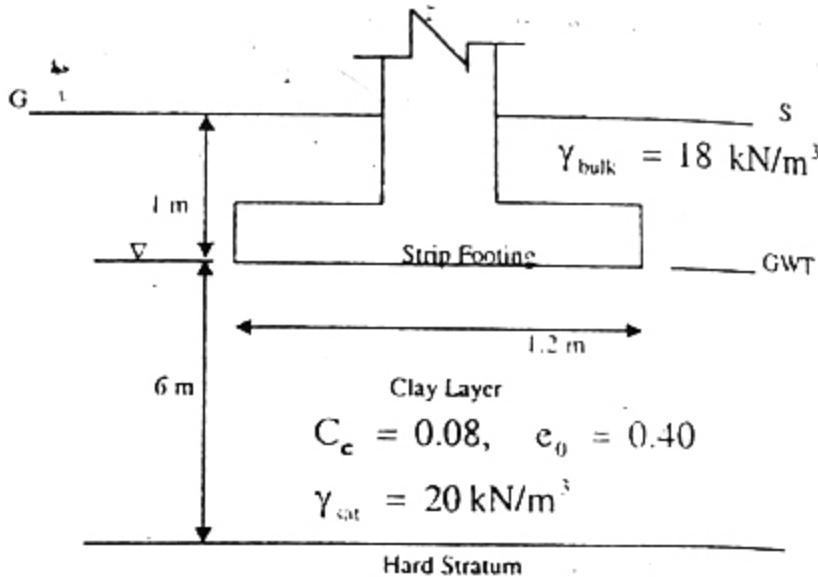
Data for Q.77-78 are given below. Solve the problems and choose correct answers.

A canal having side slopes 1:1 is proposed to be constructed in a cohesive soil to a depth of 10 m below the ground surface. The soil properties are:  $\phi_u = 15^\circ$ ,  $C_n = 12$  kPa,  $e = 1.0$ ,  $G_s = 2.65$ .

77. If Taylor's stability number,  $S_n$  is 0.08 and if the canal flows full, the factor of safety with respect to cohesion against failure of the canal bank slopes will be  
 (A) 3.7 (C) 1.0  
 (B) 1.85 (D) None of these
78. If there is a sudden drawdown of water in the canal and if Taylor's Stability Number for the reduced value of  $\phi_v$  is 0.126, the factor of safety with respect to cohesion against the failure of bank slopes will be  
 (A) 1.85 (C) 0.84  
 (B) 1.18 (D) 0.53

Data for Q.79 – 80 are given below. Solve the problems and choose correct answers.

Figure shows the geometry of a strip footing supporting the load bearing walls of a three storied building and the properties of clay layer.



79. If the pressure acting on the footing is 40 kPa, the consolidation settlement of the footing will be  
 (A) 0.89 mm (C) 89.0 mm  
 (B) 8.9 mm (D) None of these
80. If the elastic modulus and the Poisson's ratio of the clay layer are respectively  $50 \times 10^3$  kPa and 0.4 and if the influence factor for the strip footing is 1.75, the elastic settlement of the footing will be

- (A) 0.41 mm  
(B) 1.41 mm  
(C) 14.1 mm  
(D) None of these

**Data for Q.81-82 are given below. Solve the problems and choose correct answers.**

A very wide rectangular channel carries a discharge of  $8\text{m}^3/\text{s}$  per m width. The channel has a bed slope of 0.004 and Manning's roughness coefficient,  $n = 0.015$ . At a certain section of the channel, the flow depth is 1m.

81. What Gradually varied Flow profile exists at this section?  
(A)  $M_2$   
(B)  $M_3$   
(C)  $S_2$   
(D)  $S_3$
82. At what distance from this section the flow depth will be 0.9 m? (Use the direct step method employing a single step)  
(A) 65 m downstream  
(B) 50 m downstream  
(C) 50 m downstream  
(D) 65 m downstream

**Data for Q.83-84 are given below. Solve the problems and choose correct answers.**

A pipeline (diameter 0.3 m, length 3 km) carries water from point P to point R (see figure). The piezometric heads at P and R are to be maintained at 100m and 80 m, respectively. To increase the discharge, a second pipe is added in parallel to the existing pipe from Q to R. The length of the additional pipe is also 2 km. Assume the friction factor,  $f = 0.04$  for all pipes and ignore minor losses.

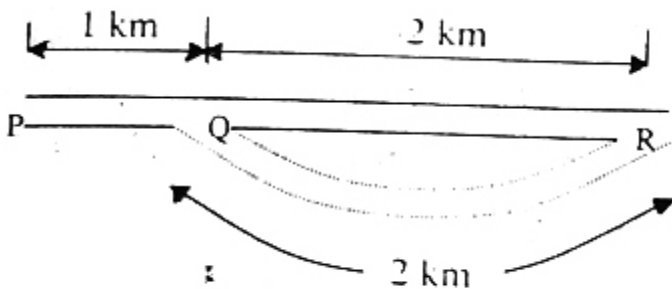


FIGURE NOT TO SCALE

83. What is the increase in discharge if the additional pipe has same diameter (0.3) m?  
(A) 0%  
(B) 33%  
(C) 41%  
(D) 67%

84. If there is no restriction on the diameter of the additional pipe, what would be the maximum increase in discharge theoretically possible from this arrangement?
- (A) 0% (C) 67%  
(B) 50% (D) 73%

**Data for Q.85-86 are given below. Solve the problems and choose correct answers.**

An average rainfall of 16 cm occurs over a catchment during a period of 12 hours with a uniform intensity. The unit hydrograph (unit depth = 1 cm, duration = 6 hours) of the catchment rises linearly from 0 to 30 cumecs in six hours and then falls linearly from 30 to 0 cumecs in the next 12 hours.  $\Phi$  index of the catchment is known to be 0.5 cm/hr. Base flow in the river is known to be 5 cumecs.

85. Peak discharge of the resulting direct runoff hydrograph shall be
- (A) 150 cumecs (C) 230 cumecs  
(B) 225 cumecs (D) 360 cumecs
86. Area of the catchment in hectares is
- (A) 97.20 (C) 9720  
(B) 270 (D) 27000

**Data for Q.87-88 are given below. Solve the problems and choose correct answers.**

A conventional Activated Sludge Plant treating  $1000 \text{ m}^3/\text{d}$  of municipal waste water disposes of its anaerobically digested sludge on relatively impervious farmland. Use the following data

- Raw Sewage : SS = 225 mg/l (70% volatile) BOD = 190 mg/l (Excess activated sludge returned to primary)
  - Primary Setting : SS – 50% removal BOD – 30% removal
  - Excess Activated Sludge : 0.4g VSS produced per g BOD applied (80% Volatile of total)
  - Anaerobic Digester : VSS reduced 50% Digested Sludge Concentration – 60% Sludge Specific Gravity – 1
  - Application on farmland :  $2 \text{ m}^3/\text{ha. d}$
87. Total volatile suspended solids to be anaerobically digested (kg/d VSS) shall be
- (A) 133 (C) 233  
(B) 168 (D) 245

88. Area requirements (ha) for disposal of the sludge on farmland shall be

- (A) 2.95 (C) 0.95  
(B) 1.95 (D) 0.55

**Data for Q.89-90 are given below. Solve the problems and choose correct answers.**

A water treatment plant treating 10 m<sup>3</sup> of water requires 20 mg/l of filter Alum, Al<sub>2</sub>(SO<sub>4</sub>)<sub>3</sub> · 18 H<sub>2</sub>O. The water has 6 mg/l of alkalinity as CaCO<sub>3</sub> (Al = 26.97, S = 32, O = 16, H = 1, Ca = 40, and C = 12).

89. Total alkalinity requirement (10<sup>6</sup> mg per day as CaCO<sub>3</sub>) matching filter Alum, shall be

- (A) 180 (C) 90  
(B) 120 (D) 60

90. Quantity of Quick Lime required (10<sup>6</sup> mg per year as CaO) shall be

- (A) 2132 (C) 4132  
(B) 3000 (D) 6132

